

# Experimental Investigation on Strength and Behavior of PSC Fiber Reinforced Beams with GGBS

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**Abstract:** The Concrete is one of the most important products which are efficiently and effectively used in the field of construction. The usage of natural aggregates in the process of production of concrete was high which lead to huge deficiency of availability of the natural aggregates. At the same time production of cements leads to more environmental pollution. Therefore, the production of concrete was altered by vast usage of admixtures and replacements for natural aggregates. In this paper M60 grade concrete is prepared by using GGBS as a partial replacement of cement which is a good strength building mineral admixture, the steel fibers were also introduced in the concrete to improve the strength parameter and for ease of work with concrete and addition of AUROMIX – 400 which is provided by FOSROC chemicals Bengaluru as super plasticizers. The concrete specimens like Cubes and Cylinders were casted and allowed to curing over a nominal curing period of 7, 14 and 28 days to know the basic mechanical properties of the concrete with the above replacements and at the same time RCC beams were also casted and cured, then post tensioned to know the flexural details of this special concrete.

**Keywords:** PSC, Fibre Reinforced Beam, GGBS, Control Beam, Steel Fibres

## 1. INTRODUCTION

In earlier era the construction method was ordinary so use of naturally available sources was sufficient to build a monument. Now we are in a stage that concrete should perform in a greater extent with all aspects required for a monument with less or no usage of naturally available material. This non-conventional concrete is so called as High Performance Concrete or High Strength Concrete. Out of different types of concrete Pre stressed Concrete is one of the high strength concrete and due to addition of steel fibers and GGBS it is more effective in strength compared to conventional concrete. The reinforcement in the concrete can be pre tensioned and post tensioned as well. The reinforcement which is tensioned after concrete is poured to the specimen is post tensioning and if reinforcement is tensioned before the concrete is poured into the specimen is pre tensioning. Cement is replaced by many admixtures in which GGBS is one of the best material which gains more energy under hydration process along with OPC contributing greater part in strength criteria. One more additive material to the concrete in matter of cracks is fibers, in which the steel fibers plays vital role in decreasing cracks and provides more flexural strength to the concrete. The chemical admixture is one of the creative products which is used for improve the workability of the concrete. Although the main aim is to reduce the usage of cement which leads in less production of cements helping in less air pollution.

## 2. Literature Review

- **Arvind Nakum et al,** They reviewed the potential industrial usage of High Strength Reinforced Concrete Incorporating GGBS and SF to improve construction strength and reduce steel reinforcing requirements. In addition to cement, blast furnace slag (GGBS) was used. It extends the life and improves the tensile strength of concrete. The research focuses on the mechanical and long-term durability qualities of cured high-strength concrete reinforced with steel fibres. As a result, they suggest that GGBS and steel fibres can be utilised in concrete as an ideal alternative for cement to increase the strength and

stiffness of concrete as well as other qualities, as well as to make concrete more cost-effective.

- **Hyeong Jae YOON & Minehiro NISHYAMA** carried out research on [3]. Pre tension technology improved flexural performance in their investigation, and SFRC is discussed. SFRC was predicted to have improved tensile qualities like as strength and stiffness. Pre-tensioned members have been utilized to reduce crack width and deflection under service loads. Steel fibre reinforced concrete pretensioned beams The volumetric fibre ratios of 0.0, 0.3, and 0.5 percent were the most important characteristics in the cyclic loading testing. According to their findings, the maximum flexural strength and initial cracking load of the beams improved by 16.4% when using steel fibre over those that did not. The maximum flexural strength of the SFRC beam is about 10% higher than that of conventional concrete. At a load of 25kN, the maximum crack widths on NC, SFRC03, and SFRC05 were 0.17mm, 0.16mm, and 0.13mm, respectively.

### 3. Summary of Literature Review

From the literature it is found that the incorporation of steel fibre in concrete helps in re-distribution stresses. Steel fibres take up the load as soon as concrete matrix cracks and reduce the propagation of cracks the incorporation of steel fibre increase the tensile strength in concrete, therefore the percentage of steel reinforcement can be reducing. It is found that the addition of GGBS up to 30% in concrete will increase the substantial gain in compressive and flexural strengths with the age. Cement can be partially substituted by GGBS up to 20% without significantly reducing flexural strength with a fibre content of 2%. In case of PSC beams, the pre stressing suppress the crack compared that of RCC beams. From the literature it has been found that the optimum percentage of steel fibre is found to as 2%.

### 4. Present Scope of the work

In this study 4 types of each 3 post tensioned beams are casted.

- a. Conventional concrete beam (CC)
- b. CC+ 10% of GGBS.
- c. CC+ 2% of steel fibre as a volume of concrete.
- d. CC+ 10% of GGBS + 2% of Steel fibre as a Volume of concrete

The beams will be tested under static loading condition and the behaviour of each type of beams can be determined and Compared the obtained results.

### 5. Objectives of the work

- To study the ultimate load carrying capacity of post-tensioned.
  - Conventional concrete (CC) beam.
  - CC + 10% of GGBS.
  - CC + 2% of steel fibre as a volume of concrete.
  - CC + 10% of GGBS + 2% of steel fibre as a volume of concrete.
- To study the flexural behaviour of SFRC post tensioned beams with GGBS.
- To study the failure modes, crack widths and crack patterns of above beams.
- To assess the cracking load, maximum load of and compared with reference (CC) beams.

### 6. Methodology

1. Collection of materials and studying basic properties
2. Mix design of M60 grade concrete using these properties.
3. Slump cone test, compressive strength, and split tensile strength tests are performed on fresh and hardened concrete, respectively.
4. Total 12 beams have rectangular cross-section of 3.4x0.23x0.3m are casted. Then the beams are cured for 28 days.
5. The beams are post tensioned until required stresses are induced. Out of which one type beam is kept as it is which acts as a reference beam (CC).
6. Then the beams are tested for 2-point static loading case.

7. The ultimate load carrying capacity, failure modes and behaviour of Post-tensioned beams are compared with that of controlled beams.

Using these test results suitable graphs is plotted. by the obtained test results conclusion are made.

## 7. Material Characterization

### 7.1 53 grade OPC

Table 1. Physical Properties of Cement

Sl. No.	Properties	Obtained Values	Requirements as per IS 12269 - 1987
1.	Fineness	2.5%	10%
2.	Soundness	1.5 mm	≠ 10mm
3.	Initial Setting Time	112 min	≧ 39 min
4.	Final Setting Time	440 min	≧ 600 min
5.	<b>Compressive Strength</b>		
	3 DAYS	39.5 N/mm <sup>2</sup>	≧ 27 N/mm <sup>2</sup>
	7 DAYS	51 N/mm <sup>2</sup>	≧ 37 N/mm <sup>2</sup>
	28 DAYS	72 N/mm <sup>2</sup>	≧ 53 N/mm <sup>2</sup>
6.	Standard Consistency	31%	-----
7.	Specific Gravity	3.08	-----

### 7.2 Fine Aggregates and Coarse Aggregates

Table 2. Physical Properties of Fine Aggregates

<b>Physical Properties of River Sand</b>		
1.	Dry Compacted bulk density	1652.18 kg/m <sup>3</sup>
2.	Loose bulk density	1441 kg/m <sup>3</sup>
3.	Specific gravity	2.73
4.	Fineness Modulus	2.89

Table 3. Physical Properties Coarse Aggregates

<b>Physical Properties</b>		
1.	Shape	Angular
2.	Dry compacted bulk density	1500 kg/m <sup>3</sup>
3.	Loose Bulk Density	1398 kg/m <sup>3</sup>
4.	Specific Gravity	2.74
5.	Fineness Modulus	6.70

### 7.3 Water

Normal potable water is used for the concrete mix. Water is the main source the hydration of the cement in concrete. Water should free from alkali content, if it there alkali aggregate reaction will takes place, as a result of this durability of concrete reduces. The pH of water must be between 6 to 8. The water free from organic impurities, silt etc. has been used in concrete.

### 7.4 Super Plasticizer

The Auromix-400 which is provided by FOSROC chemicals Bengaluru is used as super plasticizer in concrete. Auromix 400 complies with IS: 9103-1999(2007). It also complies with ASTM C494 Type G depending on the dosage.



Figure 1. Auromix – 400

### 7.5 GGBS

It is one of the byproducts obtained from iron industry. It is one of strength increasing mineral admixtures. The GGBS was provided by JSW company for the work.



**Figure 2. GGBS**

**Table 4. Physical Properties of GGBS**

Sl. No.	Parameter	Value
1.	Bulk Density	550-700 Kg/m <sup>3</sup>
2.	Bulk Density	200-350 Kg/m <sup>3</sup>
3.	Specific Gravity	2.61

### 7.6 Steel Fibre

The fibres from M/S SHAKTHIMAN MSC 6030, which meet ASTM A820 standards, were employed in this project. Fabricated from Crimped End Fibres with the following specifications, they were used in this project.



**Figure 3. Steel Fibre**

### 7.7 Post Tensioning Devices

Anchor plate is made up of cast iron material. In order to enter the pre-stressing strand into the sheathing duct at both ends of the beam, it has a route. Concrete encases this.



**Figure 4. Anchor Plate**

Wedges are used to hold the pre stressed strands. This is made out of alloy carbon steel. These wedges slitted into 3 equal parts and work like a split jaws and it contains serrations inside.



**Figure 5. Wedges**

Barrels are used to hold the wedges. These are also made out alloy carbon steel.



**Figure 6. Barrels**

The high tensioned strands from MIKI steels Bengaluru The properties of strands is given below. Ultimate tensile strength is about 1960 N/mm<sup>2</sup>. 7 ply of 12.7 mm diameter.



Figure 7. Post Tensioned Strands

Table 5. Slump variations

Mix Description	Slump Values
CC	121 mm
CFi	110 mm
CGG	115 mm
CGFi	102 mm



Figure 9. Slump variations

The addition of GGBS decreased the slump values in the tests, which were based on trial mixes. The M60 concrete mixes with admixtures fibre had a slump value of 110 when made with a water cement ratio of 0.3 and a maximum addition of 1% super plasticizer. As a result, to maintain a medium degree of workability in fibre reinforced concrete, higher dosages of super plasticizers are required.

## 8. Compression Test

In order to find out the concrete's compressive strength, this test is used. It determines the whether the concrete attains required strength at required time. It is commonly used in all the experiments to know the strength of the specimen which is going to attain from the experiment. Normally to find the compressive strength cubes of standard size (0.15x0.15x0.15) m is used. In this study of work to know the compressive strength, cubes were casted of size (0.15x0.15x0.15) m is considered and of M 60 grade.

Table 6. Comparison in Compressive Strength with respect to Curing Period and Mix Type

Sl. No.	Mix Name	Compressive Strength of cubes in N/mm <sup>2</sup>					
		7 Days		14 days		28 days	
		CC	CC+10% GG	CC	CC+ 10% GG	CC	CC+ 10% GG
1.	CC	40.81		61.68		69.84	
2.	CFi	41.55		62.28		70.29	
3.	CGG		41.85		63.53		70.83
4.	CGFi		43.13		64.27		71.64

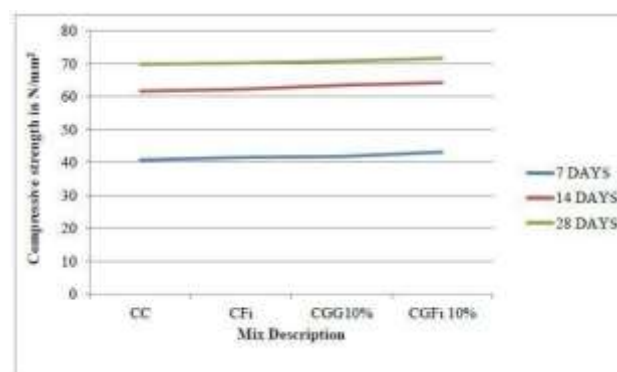


Figure 10. Comparison in compressive strength

It has been observed that with the addition of GGBS compressive strength of concrete at the age of 28 days has been increased. The increase in strength is in the range of 2.57% for SFRC with GGBS. The results showed that with the addition of fibre and GGBS in concrete the compressive strength of concrete increases compared to normal concrete.

## 9. Flexure Test

The beam specimens using for this study will be having dimension of 34x0.23x0.3m. 2 bars of 10 mm dia are provided at the bottom as tension reinforcement, 2 bars of 8 mm dia are provided at the top as hanger bars and 2L-8 mm dia stirrups spacing @ 170 mm C/C is used. Additionally, Bursting Tension reinforcement is provided at the end of the beam as end anchorage.

### 9.1 Flexural Results Obtained for Control Beam

The conventional concrete beam was kept as a control beam. The load at the first crack was 86 kN, corresponding deflection was found to be 4.962 mm. New cracks are formed as the stress increases, and as a result, the propagation occurs. And finally the ultimate load was found to be 153 kN, the corresponding deflection was 24.891mm.



Figure 10. Flexural test

Table 7. Test results of control beam

	Beam designation	Control beam
1	Load at first crack (Pcr)	86 KN
2	Centre deflection at first crack	4.962 mm
3	Ultimate load (Pu)	153KN
4	Centre deflection at ultimate load	24.891mm
5	First crack width	0.6 mm



Figure 11. Load v/s deflection

### 9.2 Flexural Results obtained for Beam with Partial Replacement of GGBS

The load at the first crack was 90 kN, corresponding deflection was found to be 4.414 mm. New cracks appeared as a result of the increased load, and crack propagation occurred as a result. And finally the ultimate load was found to be 174 kN, the corresponding crack deflection was 21.863 mm.

Table 8. Test results of beam with GGBS

	Beam designation	CC+10%GGBS
1	Load at first crack (Pcr)	90 KN
2	Centre deflection at first crack	4.414 mm
3	Ultimate load (Pu)	174 KN
4	Centre deflection at ultimate load	21.863mm
5	First crack width	0.4 mm

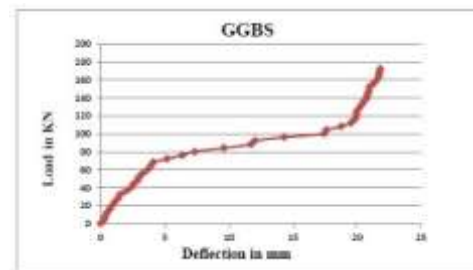


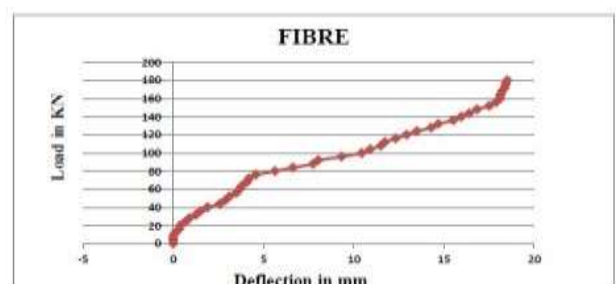
Figure 12. Load v/s Deflection (beam with GGBS)

### 9.3 Flexural Results obtained for Beam Incorporated with Steel Fibre

The load at the first crack was 96 kN, corresponding deflection was found to be 4.197 mm. New cracks appeared as the load increased. And finally the ultimate load was found to be 180 kN, the corresponding crack deflection was 18.516 mm.

Table 12. Test results of beam with Steel Fibres

	Beam designation	CC+2%Steel fibre
1	Load at first crack (Pcr)	96 KN
2	Centre deflection at first crack	4.197 mm
3	Ultimate load (Pu)	180 KN





5	Centre deflection at ultimate load	18.516 mm
6	First crack width	0.36 mm

Figure 13. Load v/s Deflection (beam with steel fibres)

#### 9.4 Flexural Results obtained for Beam Incorporated with Steel Fibre and GGBS

The load at the first crack was 104 KN, corresponding deflection was found to be 2.616 mm. New cracks appeared as the load increased. And finally the ultimate load was found to be 196 KN, the corresponding crack deflection was 15.446 mm.

Table 12. Test results of beam with Steel Fibres with GGBS

1	Beam designation	CC+2%Steel fibre+10%GGBS
2	Load at first crack (P <sub>cr</sub> )	104 KN
3	Centre deflection at first crack	2.616 mm
4	Ultimate load (P <sub>u</sub> )	196 KN
5	Centre deflection at ultimate load	15.446 mm
6	First crack width	0.3 mm

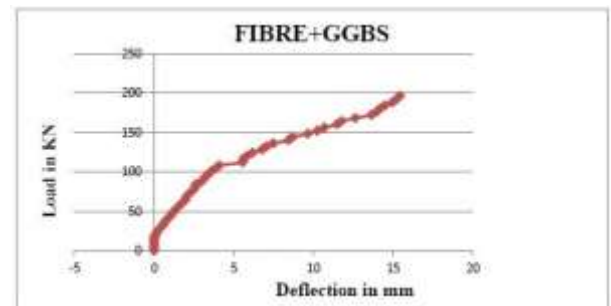


Figure 14. Load v/s Deflection (beam with steel fibers with GGBS)

### 10. Discussion Based on Results

From the obtained results it is found that, the CC post tensioned beam undergo large deflection as compared to concrete with GGBS and SFRC post tensioned beams. The first crack width is also maximum in the case of CC post tensioned beam as compared to concrete with GGBS and SFRC post tensioned beams. Also the ultimate load carrying capacity of CC post tensioned beam is less compared to GGBS and SFRC post tensioned beams.

### Conclusions

1. The ultimate load carrying capacity of conventional concrete (CC) Post-tensioned beam is 153 kN, where as in the case of GGBS Post-tensioned beam is 174 KN. i.e. the strength has increased about 13.72 %.
2. SFRC Post-tensioned Beam has a maximum load carrying capability of 180 kN, which is 17.63% higher than CC Beam. Also the ultimate load carrying capacity of SFRC with GGBS Post-tensioned beam is 196 kN, which is 28.10 % greater than CC beam
3. The first crack width for CC beam is 0.6 mm, where as in GGBS beam and SFRC with GGBS is 0.4 mm and 0.3 mm respectively.
4. Hence incorporation of steel fibre in concrete reduces the crack width and propagation of cracks.
5. Hence, the results suggest that adding GGBS to concrete increased the beam's load carrying capability significantly.
6. The environmental harm caused by cement manufacture can be reduced by substituting GGBS.
7. And the addition of fibre in concrete has serious impact on the flexural strength of the beam.

### Scope for Future Work

The same project can have carried out in analytical, and compare the result with experimental results.

1. The post tensioned beam can be tested for uniformly distributed load.
2. Since the experiment is carried out for flexural behavior, the shear behavior can also be tested.
3. Since the experiment is carried out for 10% of GGBS, with different proportions and with the different types of mineral admixtures can also be used and tested.
4. By stressing the post tensioned beams with different ratios can also be tested.

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